

## Study Report:

In 2012 Estonian Road Administration initiated a study to find out the cost effective dust control and stabilisation method for Estonian gravel roads. Ramboll Estonia AS carried out the first phase of this study. Based on this study the stabilisation methods, which will be tested in the second phase of this study in 2013-2014, were proposed. Additionally the soil stabilisation materials and methods were proposed to test for construction of dust-free on low-and high-volume roads. The goal was to make the gravel road dust-free, increase its carrying capacity and compression strength. It was also a goal to make stabilized soil water and frost resistant.

The effectiveness of various soil stabilisation materials and methods was evaluated and compared based on the results and data of visual monitoring, laboratory tests, measurements and cost benefit analysis carried out.

Two different soil stabilisation materials and methods were tested (third selection was dropped on early phase) on two 5000 m long and 6 m wide test sections on gravel road. Soil stabilisation with StabilRoad® material system and cement was performed on one test section and solution mixed with M10+50™ and LBS polymers was used with cement on second section. A half of the stabilised test section was covered with surface dressing and other part has left unsealed to better evaluate the changes of condition of stabilised gravel course.

The evenness of road surface was better on the test sections with surface dressings

As foreseen in the terms of reference of this study the test sections should be monitored how they withstand under the winter conditions

Thickness of the stabilizing layer: 35 cm

Mix ratio: 182 kg/m<sup>3</sup>

Cement: 63,7 kg/m<sup>2</sup>,

StabilRoad: 1,65 kg/m<sup>2</sup>

**Table 8. The evenness of the stabilized and coated gravel sections**

Lõik	Material	IRI mm/m		
		Direction 1	Direction 2	Average
1	Surface dressing with non-graded crushed stone aggregate 0/16	3,14	2,82	2,98
<b>2</b>	<b>Soil stabilisation with StabilRoad® material system and cement</b>	<b>2,58</b>	<b>2,81</b>	<b>2,70</b>
3	Soil stabilisation with polymers M10+50™, LBS and cement	3,66	3,23	3,45

**Table 9. Load bearing capacity of stabilized and coated gravel sections**

Test section	Material	E <sub>mod</sub> Mpa						Section average
		Direction 1			Direction 2			
		Min	Max	Ave- rage	Min	Max	Ave- rage	
1	Surface dressing with non-graded crushed stone aggregate 0/16	83	134	114	99	155	133	124
<b>2</b>	<b>Soil stabilisation with StabilRoad® material system and cement</b>	<b>459</b>	<b>704</b>	<b>586</b>	<b>456</b>	<b>623</b>	<b>551</b>	<b>569</b>
3	Soil stabilisation with polymers M10+50™, LBS and cement	274	361	311	309	399	333	322

**Table 2. Results of the measurements of soil stabilisation effectiveness characteristics**

Test section	Material	Compressive strength	Bearing capacity E <sub>mod</sub> avg	Evenness IRI avg
		Mpa	Mpa	mm/m
1	Surface dressing with non-graded crushed stone aggregate 0/16	-	124	2,98
<b>2</b>	<b>Soil stabilisation with StabilRoad® material system and cement</b>	<b>13,4</b>	<b>569</b>	<b>2,70</b>
3	Soil stabilisation with polymers M10+50™, LBS and cement	4,5	322	3,45



**Picture 20. Soil stabilization with StabilRoad® and cement**



**Picture 21. Surface after stabilization with StabilRoad® and cement**



**Picture 28. Gravel road stabilized with StabilRoad® and cement**



**Picture 29. Gravel road stabilized with StabilRoad® and cement and coated after**

**5 YEARS AFTER THE USE OF STABILIZED ROAD (temperature ranges between max. +30°C and min.-25°C) THE LABORATORY RESULTS SHOWS TO THE FOLLOWING:**

- The technical comparison showed that the bearing capacity (FWD) of road with stabilized layer of StabilRoad® has increased from 569 MPa to 599 MPa, while the load capacity of the M10 + 50™ has dropped from 322 MPa to 269 MPa
- The Laboratory comparison showed that the compressive strength of stabilized layer with StabilRoad® has increased from 13,4 MPa to 15,1 MPa, while the compressive strength of the M10 + 50™ has dropped from 4,5 MPa to 0 Mpa (the soil was the same as before the stabilization)

**Conclusions from the test:**

- 1) M10+50™ test failed, the material is initially designed to be used for fine grained sands (used during Kuwait war to prepare military airfields). Possible reason of failure – grain curve contained too little fine particles.
- 2) StabilRoad® test was technically successful and the end result is great. Obviously the Binder concentration requires optimization, to meet the required bearing capacity without overdimensioning. For gravel roads (daily traffic below 1000 vehicles/day), bearing capacity of about 300 MPa is sufficient (compressive strength not exceeding 10 MPa). In particular test, Binder concentration was not even also for a reason, that standard spreaders were used for StabilRoad®. (Stehr SBS 3000 or similar is required)

*(Another example, particularly, negative, concerns the shale ash stabilization. Ash reduces initial strength, but the increase within time is much longer and final strength higher. Experience is negative because the stronger the material is, larger monolites form with hydraulic Binder, and temperature changes are causing cracks in material. It is extremely important to optimize the Binder content so, that the cracks which appear, will be small, monolite not forming too large, but on the same, necessary compressive strength is achieved.)*

**Long term performance:**

All layers with hydraulic Binder (cement) are improving in time – afterhardening process. The process develops in logarithmic scale – yearly increase continuously reducing.

And StabilRoad® gives the basic benefits.

Estonian and Finnish experience in concrete roads is one good example – from initial 60 MPa compressive strength on road concrete, within 10 years the strength increased to over 100 MPa level. (Very different temperature and climate conditions)

## In Conclusion:

- Soil that was stabilized with StabilRoad® technology 5 years ago is still extremely durable today.
- Specially over the different seasons of the year, with very different temperatures and rainfalls.
- The road still does not require any maintenance.
- And it can be firmly asserted that maintenance will not be needed for many more years.



## TEST REPORT № 8744/13

24.08.2013 № 3-2.5/3445

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**Client:** Ramboll EestI AS

**Description of job:** Testing of stabilized soils

### **Samples:**

**Object** Põltsamaa

**Place of Sampling** Põltsamaa

**Date of taking samples and sampler**  
20.07.2013 ,

**Date of delivering samples and deliverer**  
20.07.2013 ,

AS Teede Tehnokeskus

AS Teede Tehnokeskus

**Clients marking of samples**  
Section stabilized with StabilRoad

**Reg. Number in laboratory**  
2786

### **Testing and results**

The surfaces of cores were smoothed by sawing.  
The specimens were held in a climate room in temperature  $20 \pm 2$  °C, relative humidity  $65 \pm 5$  %. The density of samples (without drying) was determined according to EVS-EN 12697-6 (method D). Compressive strength was determined according to \*EVS-EN 13286-41.

\* - not accredited by Estonian Accreditation Centre.

Laboratory designation	Specimen measurements, mm		Mass of specimen, g	Date of testing	Age of the specimens	Density, kg/m <sup>3</sup>	Crushing force, kN	Compressive strength, N/mm <sup>2</sup>	Average compressive strength, N/mm <sup>2</sup>	Breaking picture
	d	H								
4	141,7	118,0	4019	26.07	-	2160	210,0	13,3	13,4	R
5	141,9	132,6	4510			2151	223,5	14,1		R
6	142,1	142,1	4878			2165	200,9	12,7		R

\*T-81 ja T-74188 intersection

The test results apply only to the tested samples

**Position Deputy head of Laboratory Name Mark Meikas /digitally signed/**

Incomplete multiplication of the report without written permission of the testing laboratory is prohibited. Test report may not include all the background data.

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## TEST REPORT № 2060/18

20.06.2018 № 7-6.4/2285  
Page 1/1

**Client:** Megatrade OÜ – Kaido Randalu

**Description of job:** Testing of stabilized soils

**Samples:**

**Object** Põltsamaa

**Place of Sampling** Põltsamaa

**Date of taking samples and sampler**  
18.06.2018 ,

**Date of delivering samples and deliverer**  
18.06.2018 ,

Raino Piirsalu, AS Teede Tehnokeskus

Raino Piirsalu, AS Teede Tehnokeskus

**Clients marking of samples**  
-

**Reg. Number in laboratory**  
2786

**Testing and results**

The surfaces of cores were smoothened by sawing.  
The specimens were held in a climate room in temperature 20±2 °C, relative humidity 65±5 %. The density of samples (without drying) was determined according to EVS-EN 12697-6 (method D). Compressive strength was determined according to \*EVS-EN 13286-41.

\* – not accredited by Estonian Accreditation Centre.

Laboratory designation	Place of sampling	Specimen measurements, mm		Mass of specimen, g	Date of testing	Age of the specimens	Density, kg/m <sup>3</sup>	Crushing force, kN	Compressive strength, N/mm <sup>2</sup>	Average compressive strength, N/mm <sup>2</sup>	Breaking picture
		d	H								
2	~500 meters from intersection*	143,1	118,5	4397	20.06	-	2307	278,6	17,3	<b>15,1</b>	R
3	~900 meters from intersection*	143,0	118,2	4346			2293	231,4	15,7		R
4		143,2	118,7	4320			2260	201,1	14,5		R
6.1		143,2	118,6	4202			2200	191,6	12,9		R

\*T-81 ja T-74188 intersection

The test results apply only to the tested samples

**Position Deputy head of Laboratory Name Mark Meikas /digitally signed/**

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*Technical Center of Estonian Roads Ltd.*

*T-Consult*

*Megatrade OÜ*

*University of Technology*